

# Lower Granite Lock and Dam *Juvenile Bypass System (JBS) Primary & Secondary Dewatering Testing*

2019 Pacific NW Fish  
Screening and Passage  
Workshop

September 19, 2016

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# Outline

## ■ Post Construction Testing

### ▶ Hydraulic Testing

- PDS - Primary Dewatering Structure

  - ▷ ADP

  - ▷ ADV

- SDS - Secondary

  - ▷ ADV

Reference: Lower Granite JFF Commissioning Dewatering Screen Velocity Measurement Report

### ▶ Biological Testing

References: PNNL 28331 or 29052, 28461 (2018)



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# NOAA Fisheries Criteria

## 1.1 NOAA Fisheries Dewatering Screen Design Guidance

1.1.1 Horizontal screen/Incline Screen (floor screen in this case) sweep velocity > 2.5 ft/s for horizontal screens and less than 3 ft/s for incline screens.

PDS – Since the primary dewatering floor screen has a slope of -6%, it is closer to a horizontal screen than an inclined screen, therefore coordination with NOAA-Fisheries resulted in a sweeping velocity criteria of greater than 2.5 ft/s and less than 6 ft/s with a goal not to decelerate along the screen.

SDS – The secondary dewatering floor screen is truly horizontal and therefore is only required to be greater than 2.5 ft/s. Sweeping velocity exceed this by at least at least 3 times and is not reported in this document.

1.1.2 Vertical screen sweep velocity: optimally between 0.8 ft/s and 3 ft/s, and must not decrease along screen. Negotiation with NOAA Fisheries changed the 3 ft/s maximum magnitude at the wall screens where the PDS constricts up to 5 ft/s.

1.1.3 Approach (through screen) velocity < 0.4 ft/s.

Approach (through screen) velocity < 0.4 ft/s.

No deceleration.

Minimize holding.

## 1.2 Post-construction Evaluation

1.2.1 Confirm approach and sweep velocities across entire screen face.

1.2.2 Each measurement must represent < 0.05 of total through-screen discharge, and be located at the center point of each grid section, as close as possible to screen face.

1.2.3 No individual approach velocity > 0.44 ft/s.



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# Flow Quantifiers

1.

Acoustic Doppler Profiler (ADP)

3. Weirs

2.

Acoustic  
Doppler  
Velocimeter  
(ADV)

# Primary Dewatering Structure (PDS) Conditions

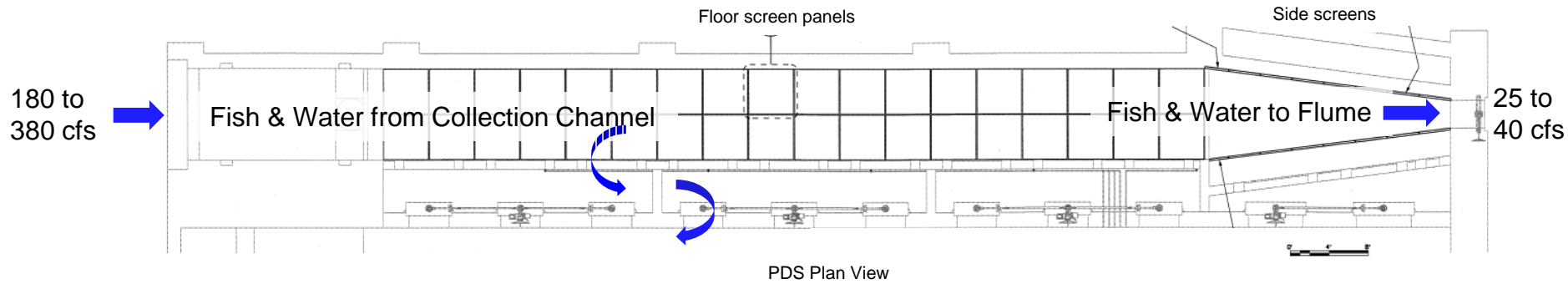
	Norm	Max	Max Test Conditions	Norm	High
	#1	#2	#3	#4	#5
Date	3/30/18	4/4/18	10/23/18	11/2/18	11/9/18
Estimated Inflow (cfs)	280	350	330	303	308
Forebay Avg. (fmsl)	734.9	735.3	736.6	736.6	735.7
Forebay Min. (fmsl)	734.8	735.1	736.5	736.5	735.6
Forebay Max. (fmsl)	735.0	735.4	736.8	736.7	735.7
Units Running Avg.	3	2	2	2	1.6
Powerhouse Flow Avg. (kcfs)	54	30	26	26	23
14" orifices open	18	18	17	15	17
10" orifices open	0	7	4	3	3
Water Supply ON/OFF	ON	ON	ON	OFF	OFF
PDS Exit Flow Setpoint (cfs)	30	25	35	35	35
Weir Position (relative)	Level	Level	B low	Level	B low D high
Dewatered Q (cfs)	250	325	295	268	273



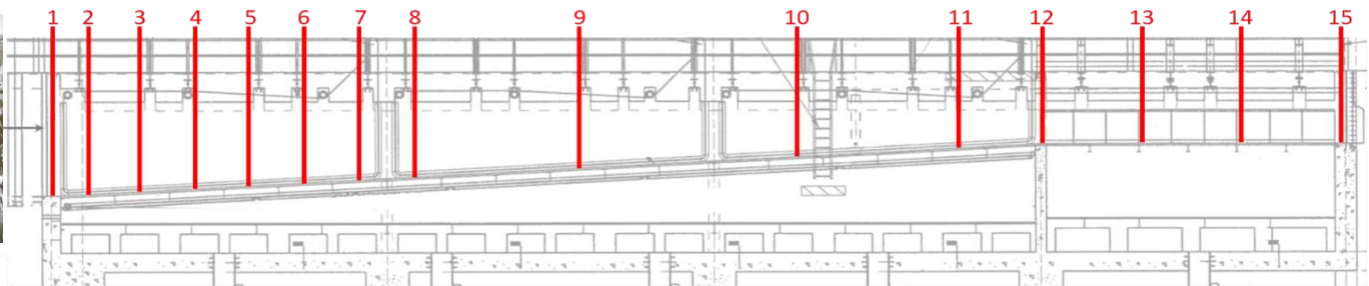
# PDS Velocimetry Overview

## Water Budget Approach

PDS



ADP

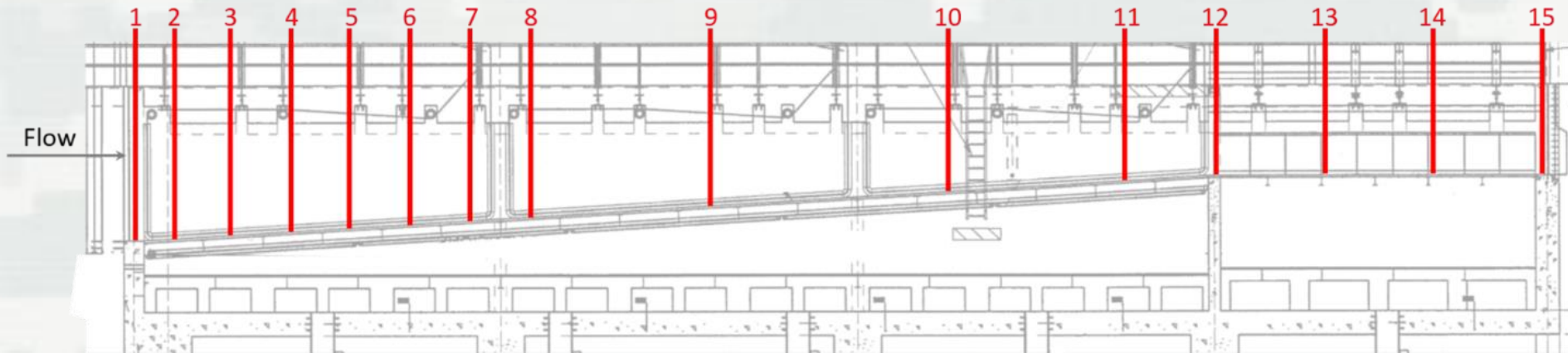


ADV





Date	Condition	V <sub>approach</sub> (ft/s)											
		1 - 2	2 - 3	3 - 4	4 - 5	5 - 6	6 - 7	7 - 8	8 - 9	9 - 10	10 - 11	11 - 12	12 - 15
3/30	Norm w/ WS	-0.24	0.53	0.22	0.28	0.15	0.23	0.20	0.23	0.19	0.22	0.37	0.39
4/4	Max w/ WS	0.34	0.28	0.30	0.12	0.31	0.44	0.24	0.28	0.27	0.28	0.48	0.51
10/23	Max w/ WS, B dwn	0.04	0.09	0.26	0.33	0.25	0.29	0.28	0.25	0.24	0.28	0.65	0.37
11/2	Norm no WS	0.06	0.07	0.47	0.19	0.19	0.20	0.33	0.22	0.20	0.24	0.42	0.40
11/9	High no WS, B dwn D up	0.06	0.26	0.23	0.29	0.14	0.22	0.15	0.28	0.22	0.24	0.48	0.39



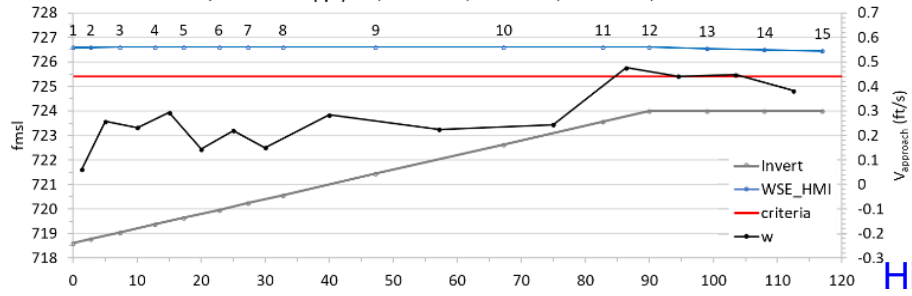
ADP cross section measurement locations.

NOAA Fisheries target  $V_{\text{approach}}$ : 0.4 ft/s



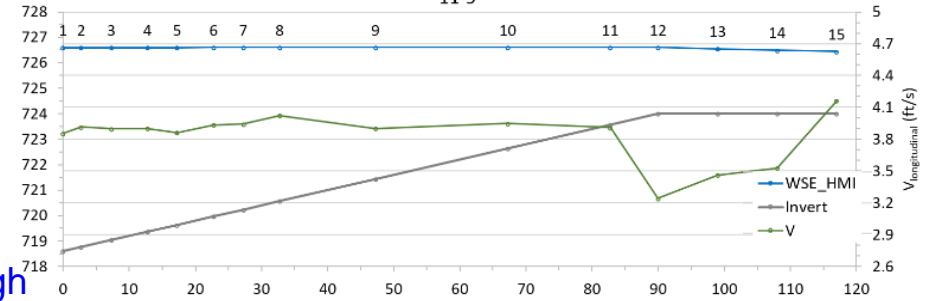
# ADP Velocity Longitudinal Profiles

$V_{\text{approach}}$   
11/9: Water Supply Off, 308 cfs in, 31 cfs out, B lowered, D raised

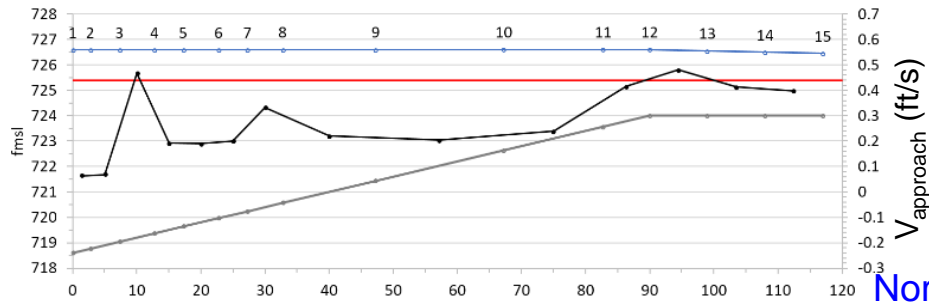


High

$V_{\text{longitudinal}}$   
11-9

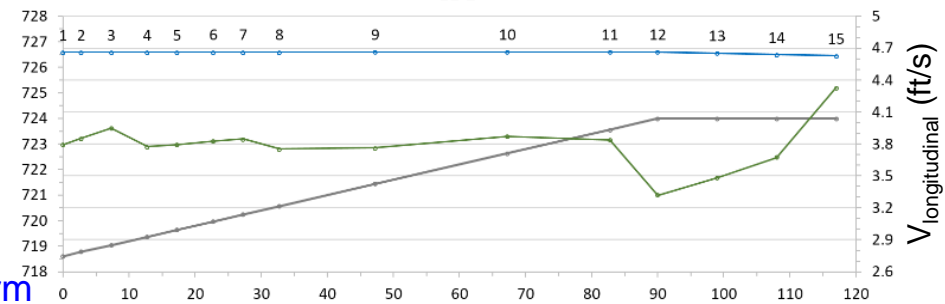


11-2: Water Supply Off, 303 cfs in, 32 cfs out; Weirs Flat

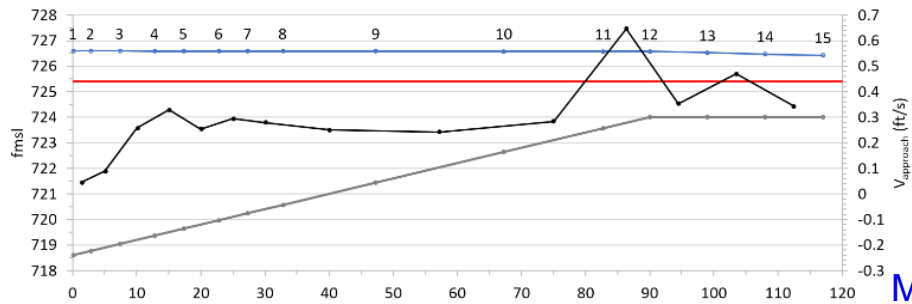


Norm

11-2

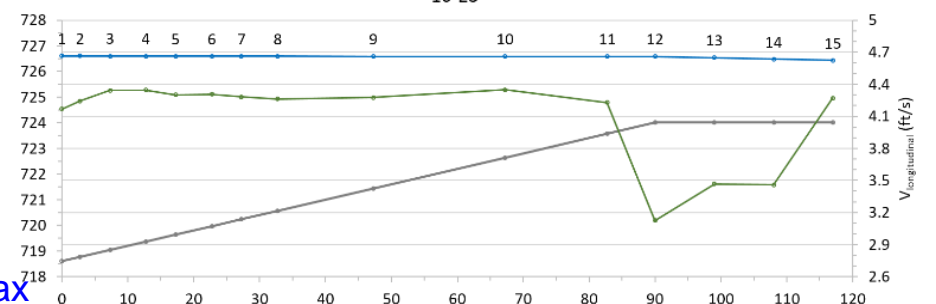


10-23: Water Supply On, 333 cfs in, 31 cfs out, B Lowered



Max

10-23

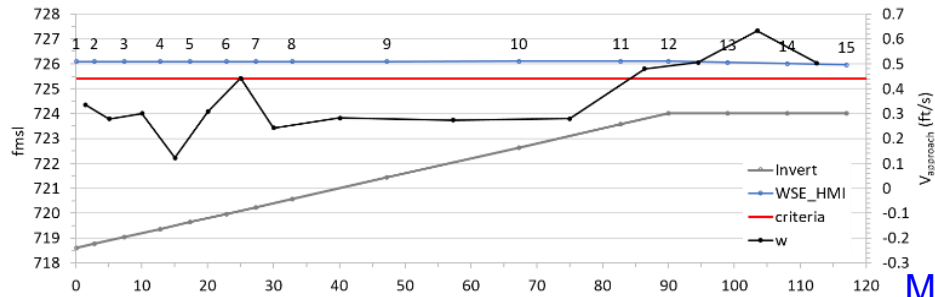




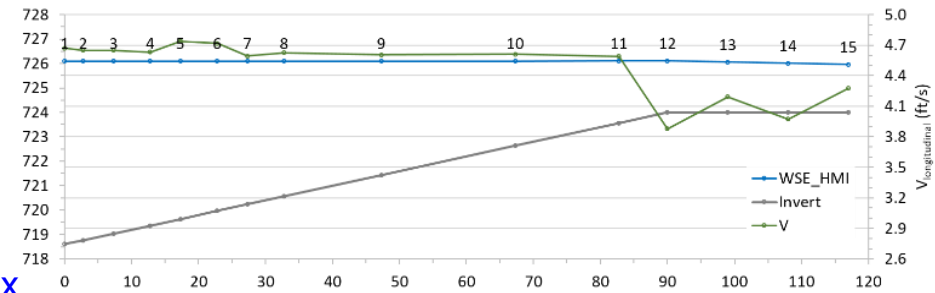
# ADP Velocity Longitudinal Profiles

 $V_{\text{approach}}$ 

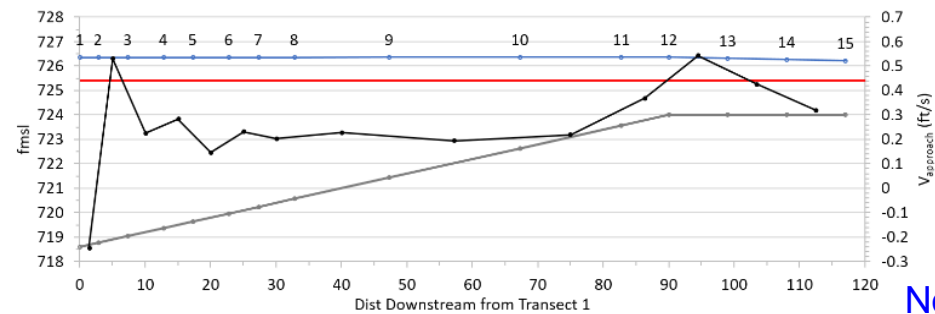
4-4: Water Supply On, 350 cfs in, 25 cfs out; Weirs Flat


 $V_{\text{longitudinal}}$ 

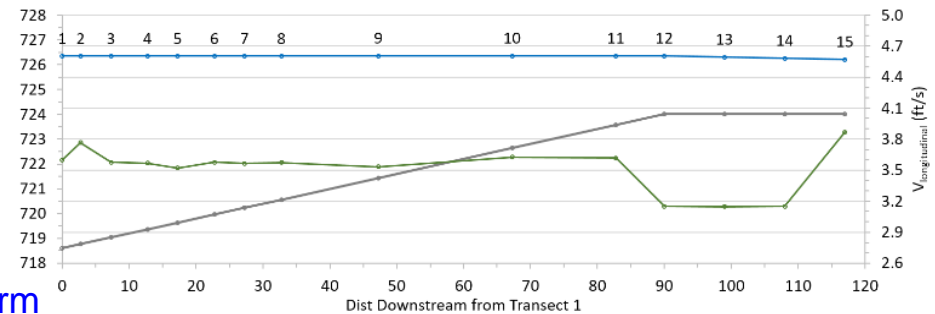
4-4



3-30: Water Supply On, 279 cfs in, 26 cfs out; Weirs Flat

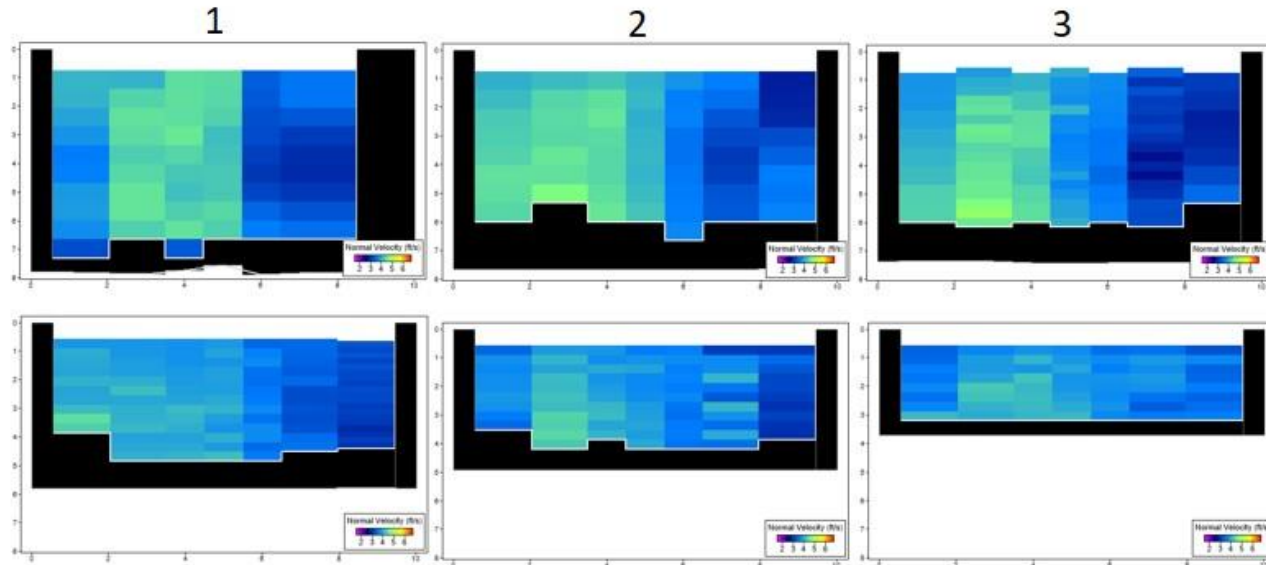


3-30

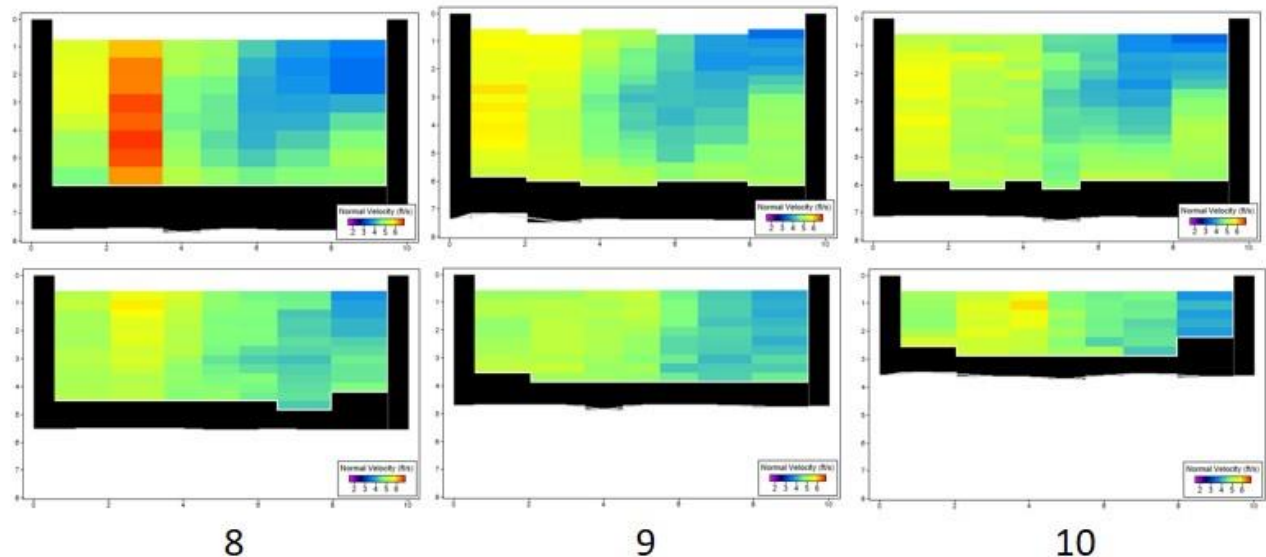


# ADP Velocity Cross Section Plots

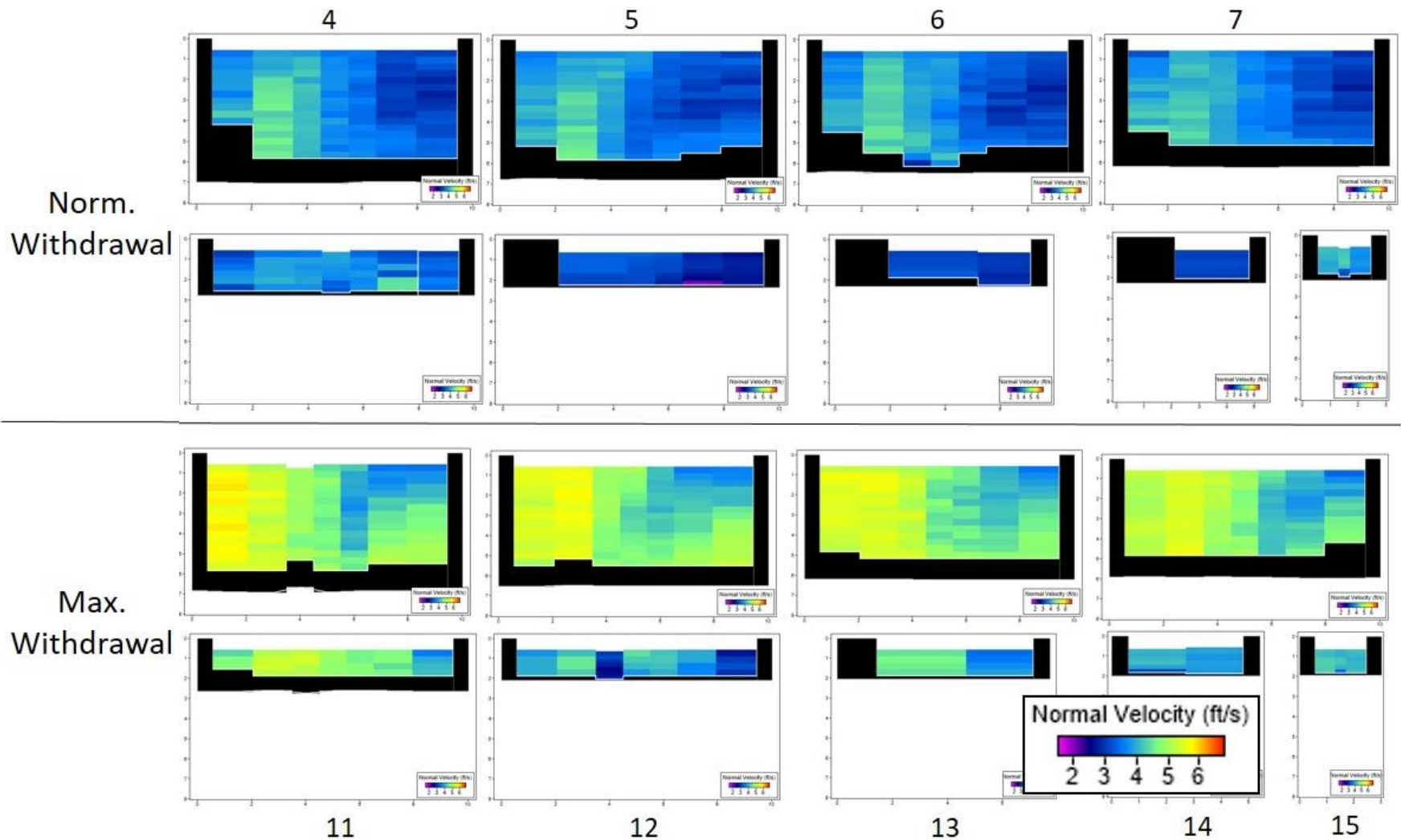
Norm.  
Withdrawal



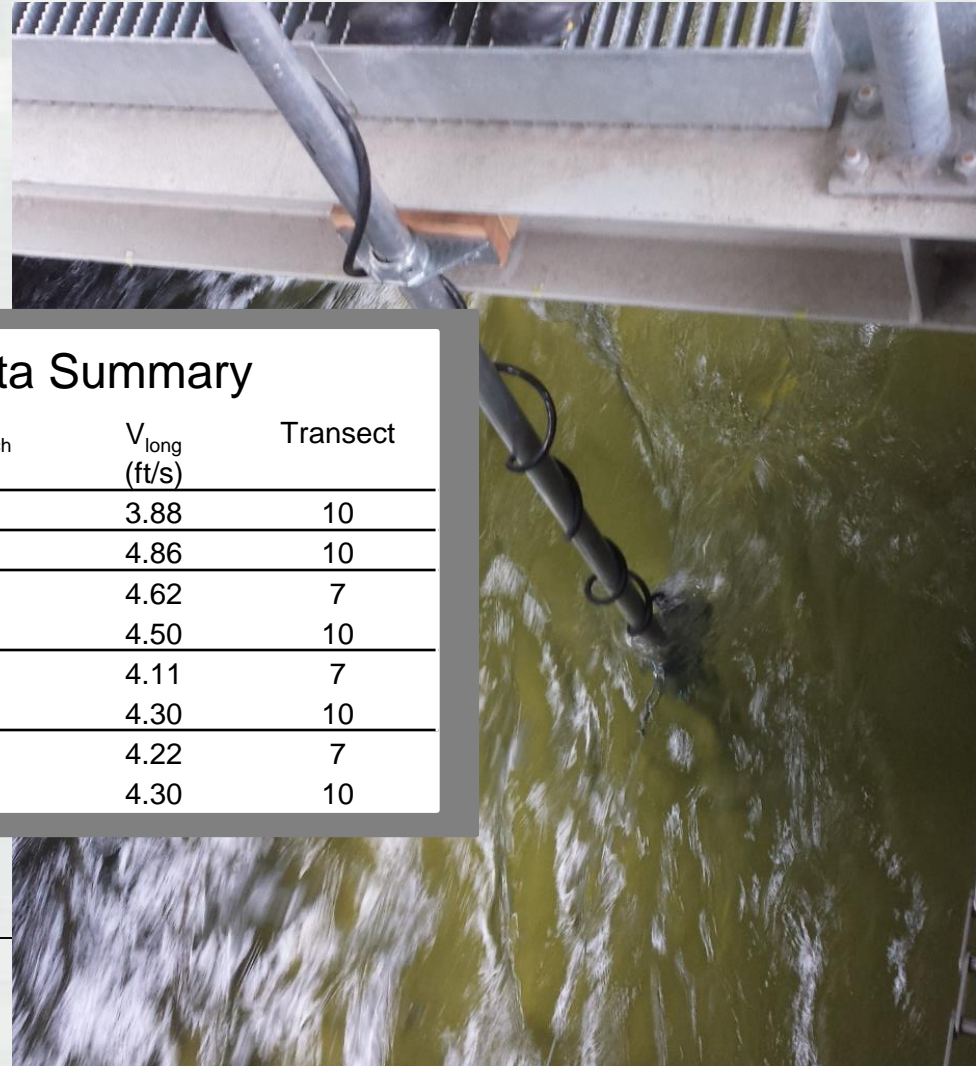
Max.  
Withdrawal



# ADP Velocity Cross Section Plots







2018 Date	V <sub>approach</sub> (ft/s)	V <sub>long</sub> (ft/s)	Transect
3/30	-0.17	3.88	10
4/4	-0.28	4.86	10
10/23	-0.23	4.62	7
	-0.11	4.50	10
11/2	-0.06	4.11	7
	-0.04	4.30	10
11/9	-0.05	4.22	7
	0.12	4.30	10

**PDS  $Q_{ADP}$  vs  $Q_{weir}$  Withdrawal; Water Supplied to JFF**

		Water Supply On			Water Supply Off	
		Norm	Max	Max. B lowered	Norm	Norm. B Raised, D Lowered
		3/30	4/4	10/23	11/2	11/9
$Q_{weir}$	a.m.	187	283	246	275	278
	p.m.	196	284	238	265	293
$Q_{ADP}$		253	325	302	271	278
Water Supplied to JFF	a.m.	66	42	56	-4	0
	p.m.	57	41	64	6	-15

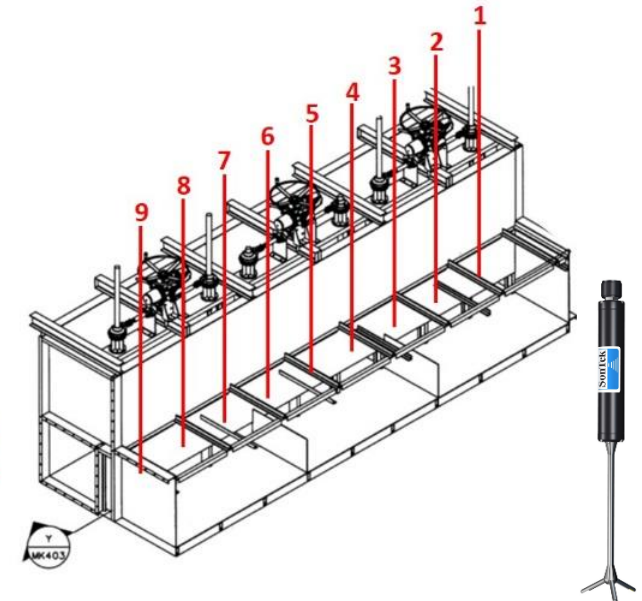
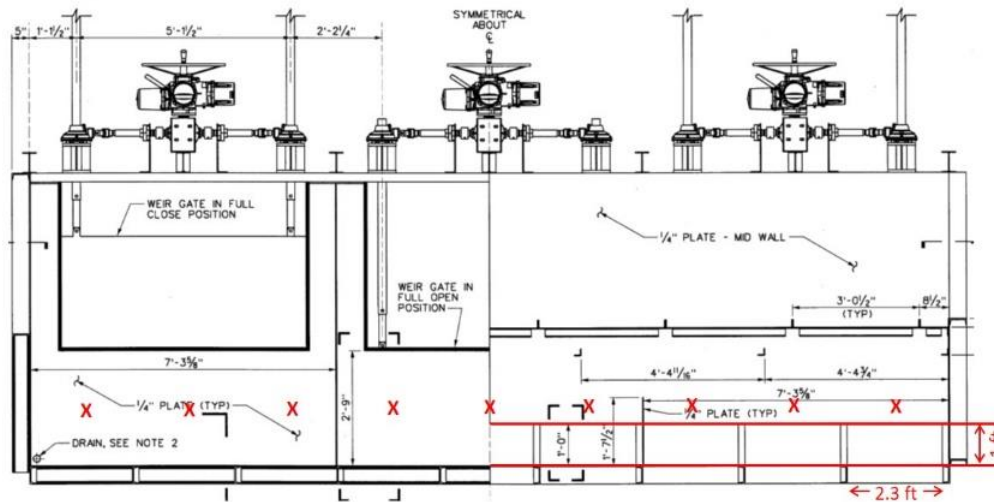
$$Q_{weir} = \frac{2}{3} C(b - 0.2H)H^{3/2}\sqrt{2g}$$

$$C = 0.602 + 0.083 \frac{H}{W}$$



# Secondary Dewatering Structure (SDS)

SDS



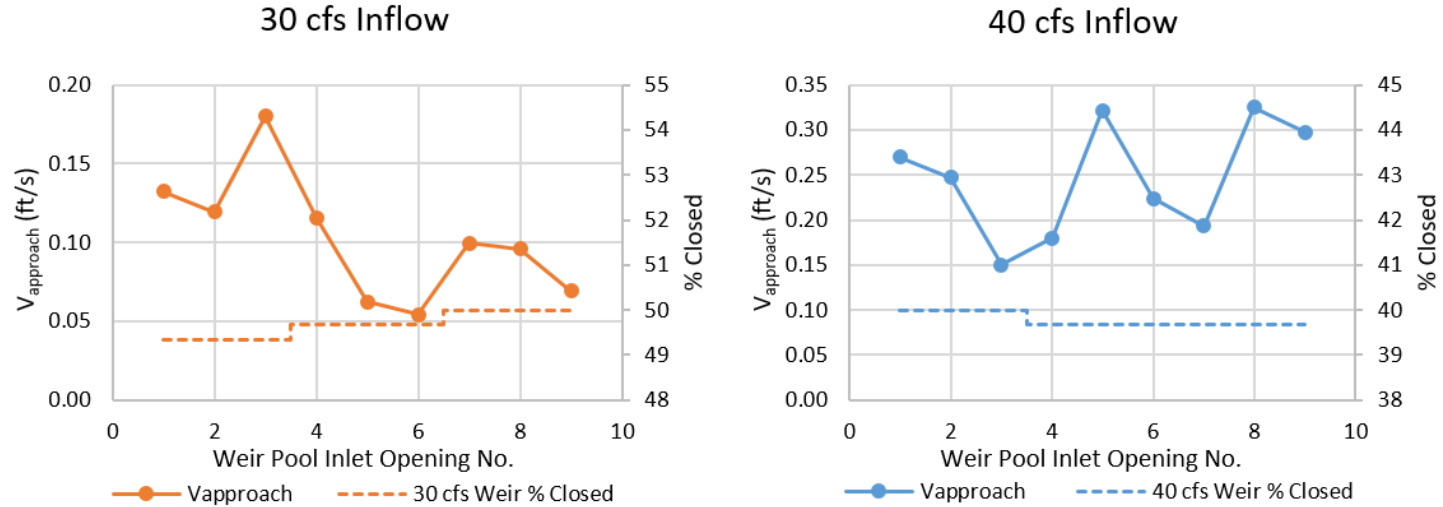
- SDS weirs adjusted so downstream hydraulic conditions (mostly the location of the hydraulic jump) deemed acceptable by project biologist
- Then ADV measurements

$Q_{ADV}$  (cfs; velocity x cross section area of opening) Through Each Weir Pool Inlet Opening

$Q_{ADV}$ (cfs)	9	8	7	6	5	4	3	2	1
30 cfs Inflow	0.48	0.66	0.69	0.37	0.43	0.80	1.24	0.82	0.91
40 cfs Inflow	2.06	2.24	1.33	1.55	2.22	1.24	1.04	1.71	1.86



# SDW $Q_{ADV}$ vs $Q_{weir}$



SDW weir opening velocity magnitude and weir % closure.

## $Q_{ADV}$ vs $Q_{weir}$

Inflow (cfs)	30
$Q_{weir}$	(cfs)
Weir No.	
1	3.8
2	2.4
3	0.5
$Q_{ADV}$	6.4
Total $Q_{weir}$	6.7



# Biological Testing

Travel Time

Max Flow: 3:19 (min:s) to PDS endgate

Norm Q: 4:6 to PDS endgate

Endgate to Separator at JFF: 2:46

## JBS Descaling for Yearling Chinook (hatchery)

Year	%
2013	14.4
2014	9.2
2018	5.6

PNNL-28461 Post-Construction Evaluation of the Upgraded Juvenile Bypass System at Lower Granite Dam; Final Technical Report; July 2019

## 2018 Survival Compared to Past Studies

Year	% Increase in Survival	Class
2005	1	Yearling
2006	5	Yearling
2007	15	Subyearling

PNNL-29052 or 28331 Survival and passage of Yearling and Subyearling Chinook Salmon and Juvenile Steelhead at Lower Granite Dam; Aug 2019

Photo (right) showing a dead tagged fish attached to the deep-water release mechanism prior to release into the outflow from Turbine Unit 1 at Lower Granite Dam. (PNNL-28325)



# Summary

Worked:

- ADP water budget approach for an estimated  $V_{\text{approach}}$

Did not work quite as well:

- ADV point measurements in PDS
- ADP reflection (<2 ft depth) flat stainless steel channel bed



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# Questions?

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# Survival and Passage of Yearling and Subyearling Chinook Salmon and Juvenile Steelhead at Lower Granite Dam, 2018

## Final Technical Report

August 2019

Pacific Northwest National Laboratory

(also known as PNNL-28331 – a different number is assigned to the different drafts. This report has not been officially finalized yet.)

Table E.5. Survival Estimates by Route for CH1 at Lower Granite Dam During Spill

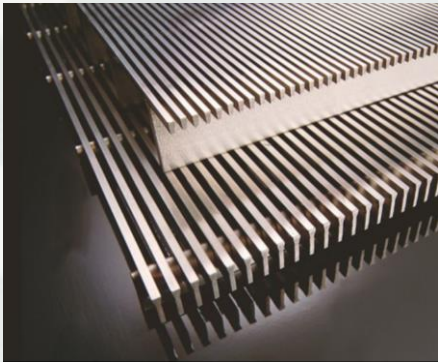
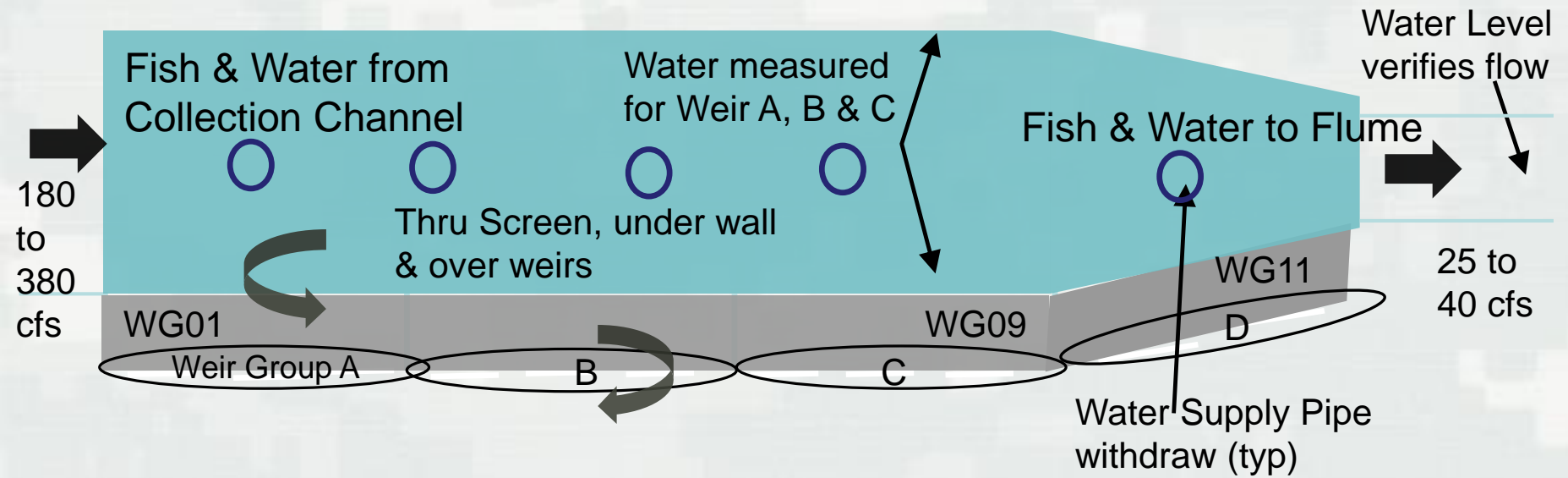
Year	Spill Operation	Spillway	RSW	JBS	Turbine	Source
2003	24-hr Spill RSW+12 kcfs	-	0.980 (0.023)	-	-	Plumb et al. 2004
2005	Involuntary Spill	0.905 (0.057)	0.982 (0.016)	0.097 (0.018)	1.011 (0.169)	Perry et al. 2007
2006	24-hr Spill RSW+12 kcfs	0.970 (0.018)	0.985 (0.016)	0.987 (0.014)	0.815 (0.086)	Beeman et al. 2008
2018	Court-Ordered Gas Cap Spill	0.9521 (0.0244)	0.9855 (0.0172)	0.9961 (0.0158)	0.8779 (0.0599)	This Study (VIPRE)
		0.9898 (0.0102)	1.0016 (0.0360)	1.0001 (0.0264)	0.8697 (0.0604)	This Study (ViRDcT)

Table E.6. Survival Estimates by Route for STH at Lower Granite Dam During Spill

Year	Spill Operation	Spillway	RSW	JBS	Turbine	Source
2006	24-hr Spill RSW+12 kcfs	0.985 (0.013)	0.952 (0.022)	0.955 (0.017)	0.879 (0.082)	Beeman et al. 2008
2018	Court-Ordered Gas Cap Spill	1.0003 (0.0119)	0.9843 (0.0141)	1.0111 (0.0087)	0.8804 (0.0715)	This Study (VIPRE)
		1.0002 (0.0153)	0.9937 (0.0063)	1.0000 (0.0124)	0.9076 (0.0626)	This Study (ViRDcT)

Table E.7. Survival Estimates by Route for CH0 at Lower Granite Dam During Spill

Year	Spill Operation	Spillway	RSW	JBS	Turbine	Source
2007	24-hr Spill RSW+12 kcfs	0.811 (0.044)	0.922 (0.023)	0.853 (0.042)	0.872 (0.067)	Puls et al. 2008
2018	Court-Ordered Gas Cap Spill	0.8456 (0.0321)	0.9655 (0.0230)	1.0023 (0.0277)	0.9949 (0.0306)	This Study (VIPRE)
		0.8450 (0.0323)	0.9654 (0.0234)	1.0022 (0.0280)	0.9949 (0.0309)	This Study (ViRDcT)



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Table 13 Q<sub>ADV</sub> through each opening separately for 30 cfs.

Opening (No.)	V Mag (ft/s)	Q (cfs)	Area (ft <sup>2</sup> )	V <sub>approach</sub> (ft/s)	Weir Opening %
1	0.40	0.91	6.91	0.13	50.67
2	0.36	0.82		0.12	
3	0.54	1.24		0.18	
4	0.35	0.80		0.12	50.33
5	0.19	0.43		0.06	
6	0.16	0.37		0.05	
7	0.30	0.69		0.10	50.00
8	0.29	0.66		0.10	
9	0.21	0.48		0.07	

ADV Data @ SDS

Table 15 Q<sub>ADV</sub> through each opening separately for 40 cfs.

Opening (No.)	V Mag (ft/s)	Q (cfs)	Area (ft <sup>2</sup> )	V <sub>approach</sub> (ft/s)	Weir Opening %
1	0.81	1.86	6.91	0.27	60.00
2	0.75	1.71		0.25	
3	0.45	1.04		0.15	
4	0.54	1.24		0.18	60.33
5	0.97	2.22		0.32	
6	0.68	1.55		0.22	
7	0.58	1.33		0.19	60.33
8	0.98	2.24		0.32	
9	0.90	2.06		0.30	

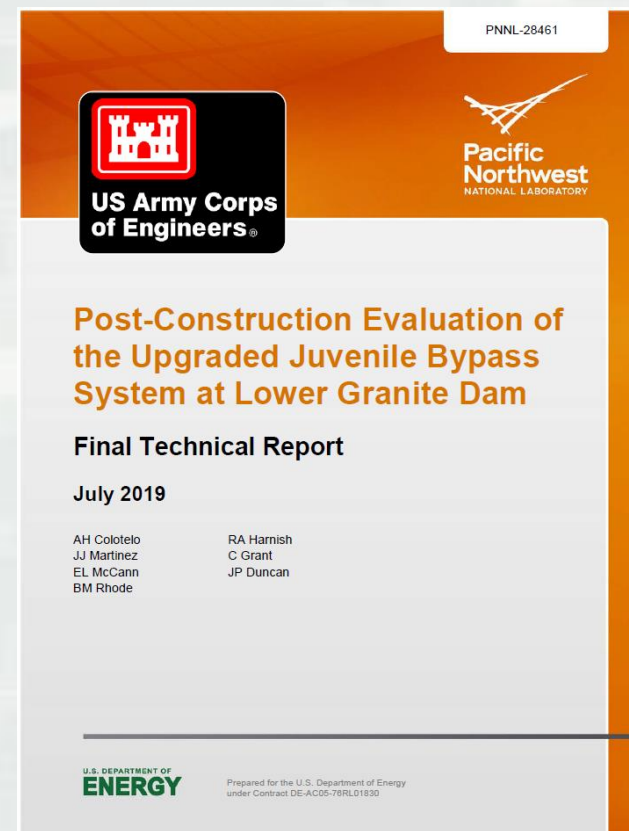
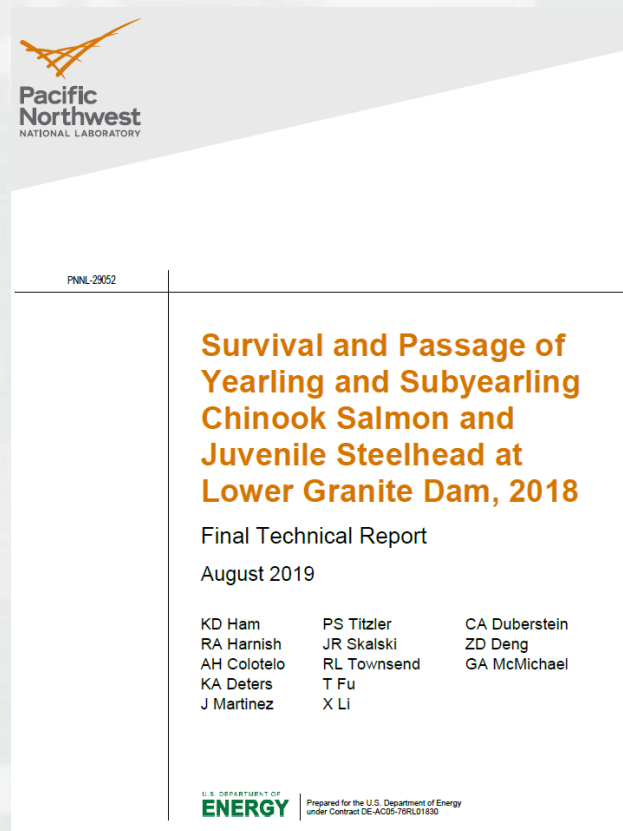
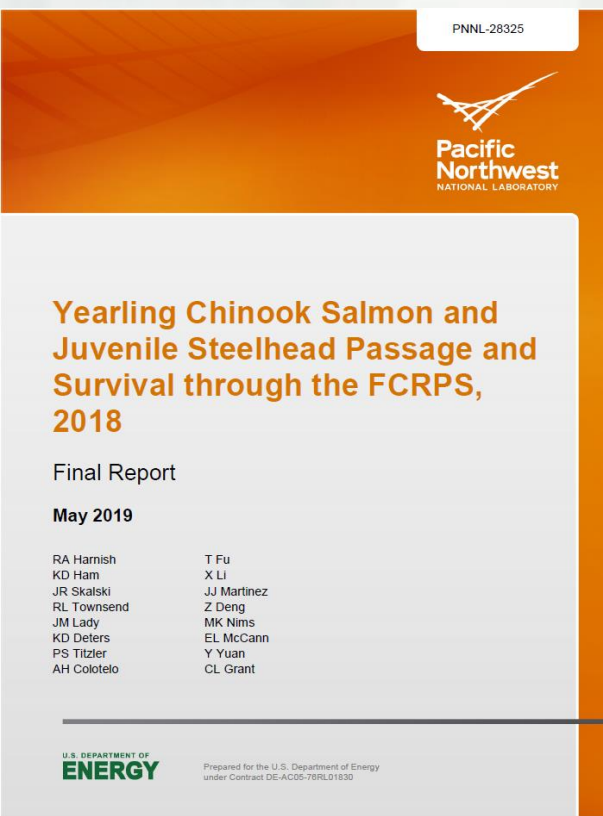


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		V <sub>approach</sub> (ft/s)											
Date	Condition	1 - 2	2 - 3	3 - 4	4 - 5	5 - 6	6 - 7	7 - 8	8 - 9	9 - 10	10 - 11	11 - 12	12 - 15
3/30	Norm w/ WS	-0.24	0.53	0.22	0.28	0.15	0.23	0.20	0.23	0.19	0.22	0.37	0.39
4/4	Max w/ WS	0.34	0.28	0.30	0.12	0.31	0.44	0.24	0.28	0.27	0.28	0.48	0.51
10/23	Max w/ WS, B dwn	0.04	0.09	0.26	0.33	0.25	0.29	0.28	0.25	0.24	0.28	0.65	0.37
11/2	Norm no WS	0.06	0.07	0.47	0.19	0.19	0.20	0.33	0.22	0.20	0.24	0.42	0.40
11/9	High no WS, B dwn, D raised	0.06	0.26	0.23	0.29	0.14	0.22	0.15	0.28	0.22	0.24	0.48	0.39







## Relevant PNNL Reports





## **Approach Velocity Measurement Issue**

### **Traditional Method:**

- **Approach Velocity Measured with ADV Meter a Few Inches from Screen Face**
- **Can Work Well in Low-Velocity Channel Flow Environments**

### **Issue:**

- **Many Recent Designs Use High-Velocity Channel Flow to Trap Fish**
- **This Can Compromise Ability to Accurately Measure Approach Velocity**

### **Solution:**

- **Alternate Method to Reasonably Calculate Estimates of Approach Velocities**

## Concerns with traditional approach

### Location:

- If Perpendicular Velocity is High then Screen Flow is NOT at Meter Location
- Example: Meter 3 Inches From Screen

Approach Velocity = 0.40 ft/s

Perpendicular Channel Velocity = 4.0 ft/s (10 Times Approach)

Approach Flow Travels Downstream 30 Inches before Reaching Screen

At Trapping Velocity of 7 to 8 ft/s Problem Even Worse

### □ Meter Positioning:

- Accurate Meter Positioning Becomes Nearly Impossible



**Scenario 1 - Input Parameters**

Assumed $V_x$	8 fps
Assumed $V_y$	5.5 fps
Resultant Velocity ( $V_r$ )	8.14 fps
Resultant X-Y Vector Angle ( $\alpha$ )	42.51 deg
$V_a$ - Actual	0.35 fps
$V_s$ - Actual	8.13 fps

**Analysis 1**

Probe Alignment ( $\delta$ ) (deg from true)	$V_a$ - Meas (fps)	$V_s$ - Meas (fps)	% Error	
			$V_a$	$V_s$
-5	1.06	8.07	200%	-0.8%
-2	0.64	8.11	80%	-0.2%
-1	0.50	8.12	40%	-0.1%
-0.25	0.39	8.13	10%	0.0%
0	0.35	8.13	0%	0.0%
0.25	0.32	8.13	-10%	0.0%
1	0.21	8.14	-40%	0.1%
2	0.07	8.14	-80%	0.1%
5	-0.36	8.13	-201%	0.0%

**Scenario 2 - Input Parameters**

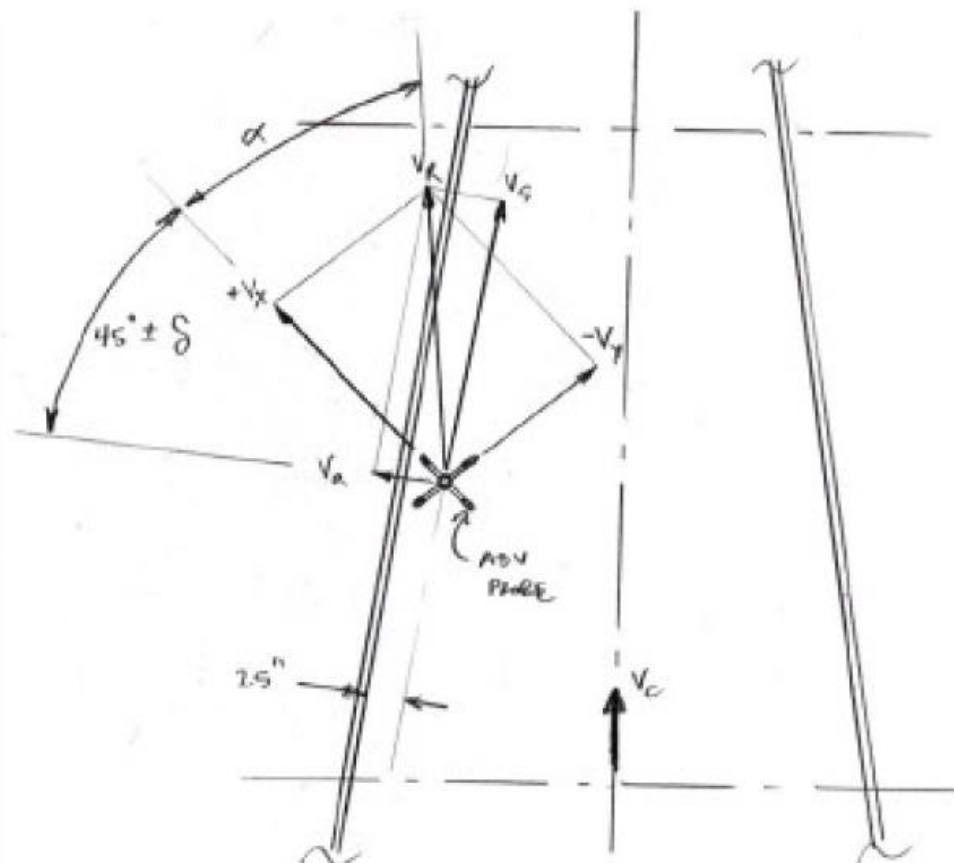
Assumed $V_x$	2 fps
Assumed $V_y$	1.5 fps
Resultant Velocity ( $V_r$ )	2.50 fps
Resultant X-Y Vector Angle ( $\alpha$ )	36.87 deg
$V_a$ - Actual	0.35 fps
$V_s$ - Actual	2.47 fps

**Analysis 2**

Probe Alignment ( $\delta$ ) (deg from true)	$V_a$ - Meas (fps)	$V_s$ - Meas (fps)	% Error	
			$V_a$	$V_s$
-5	0.57	2.43	61%	-1.6%
-2	0.44	2.46	24%	-0.6%
-1	0.40	2.47	12%	-0.3%
-0.8	0.39	2.47	10%	-0.2%
0	0.35	2.47	0%	0.0%
0.80	0.32	2.48	-10%	0.2%
1	0.31	2.48	-12%	0.2%
2	0.27	2.49	-24%	0.4%
5	0.14	2.50	-61%	0.9%

# Meter Positioning Accuracy

(Courtesy of Alden Laboratories)



$$V_a = V_r \cos(\alpha + 45 \pm \delta)$$

## Method for Calculating Approach Velocity Estimates

- Requirements:
  - Screen Divided into Independently Controlled Sections
  - Control Baffles Provide Evenly Distributed Flow over Screen Area
- Approach:
  - Preset Baffles to Design Conditions (if possible)
  - Measure Channel Flow at Discrete Locations along Channel
  - Adjust Overall Flow to Meet (or approximate) Targets
  - Difference in Flow Measurements is Screen Flow Leaving Channel
  - Measure Head Difference Across Individual Screen Panels
  - Calculate Approach Velocity Estimates Based on Heads





# Calculation of Approach Flow Estimates

**Flow Through a Screen Section is Defined by Following Governing Equation:**

$$Q_p = C * A_p * P * (2G)^{0.5} * H^{0.5}$$

**Where:**  $Q_p$  = Screen Panel Flow

$C$  = Screen Orifice Flow Coefficient

$A_p$  = Submerged Area of Screen Panel

$P$  = Open Area Porosity of Screen Material

$G$  = Gravity Acceleration

$H$  = Hydraulic Headloss across the Screen Panel

**Given Identical Screen Material, and Near Constant Channel Environment Conditions:**

**The Flow Coefficients, Screen Porosities, and Gravity are Essentially Constants**

**Therefore:**  $Q_p = Z * A_p * H^{0.5}$       where the Correction Factor  $Z = C * P * (2G)^{0.5}$

**So the Total Screen Flow Out of the Channel Section :**

$$Q_T = Z * (A_1 * H_1^{0.5} + A_2 * H_2^{0.5} + A_3 * H_3^{0.5} + A_4 * H_4^{0.5} + A_5 * H_5^{0.5} + A_6 * H_6^{0.5} + A_7 * H_7^{0.5} + A_8 * H_8^{0.5})$$

# Calculation of Approach Flow Estimates

*Channel Section P-2 (Between Transects 2 and 3)*

<b>Primary Section P-2</b>	Starboard				Port				Total
Individual Screen Panel Approach Velocity Estimates	Screen Panel P5-A	Screen Panel P5-B	Screen Panel P7-A	Screen Panel P7-B	Screen Panel P6-A	Screen Panel P6-B	Screen Panel P8-A	Screen Panel P8-B	Screen Flow (cfs) <b>198.86</b>
Screen Area (ft <sup>2</sup> ):	67.95	67.98	67.95	68.02	67.85	67.93	67.95	67.98	Correction Factor 2.48
Measured Screen Headloss (in):	0.20	0.30	0.30	0.30	0.20	0.25	0.30	0.25	
Screen Headloss (ft):	0.02	0.03	0.03	0.03	0.02	0.02	0.03	0.02	<u>Totals</u>
Square Root of Headloss (ft <sup>1/2</sup> ):	0.13	0.16	0.16	0.16	0.13	0.14	0.16	0.14	
Area*Square Root of Headloss (ft <sup>2.5</sup> ):	8.77	10.75	10.74	10.75	8.76	9.81	10.74	9.81	80.14
Screen Panel Flow (cfs)	21.77	26.67	26.66	26.69	21.73	24.33	26.66	24.35	198.86
Screen Panel Approach Velocity (ft/s):	<b>0.32</b>	<b>0.39</b>	<b>0.39</b>	<b>0.39</b>	<b>0.32</b>	<b>0.36</b>	<b>0.39</b>	<b>0.36</b>	

The Sum of the Screen Areas times the Square-roots of the Heads = 80.14

Multiplying 80.14 times 2.48 Results in the Measured Flow of 199 cfs

As a Check:  $Z/(2G)^{0.5}/P = C$   $2.48/64.4^{0.5}/0.43 = 0.70$

Which is a Reasonable Screen Flow Coefficient for These Conditions

# Approach Velocity Measurement Issue

## Traditional Method:

- Approach Velocity Measured with ADV Meter a Few Inches from Screen Face
- Can Work Well in Low-Velocity Channel Flow Environments

## Issue:

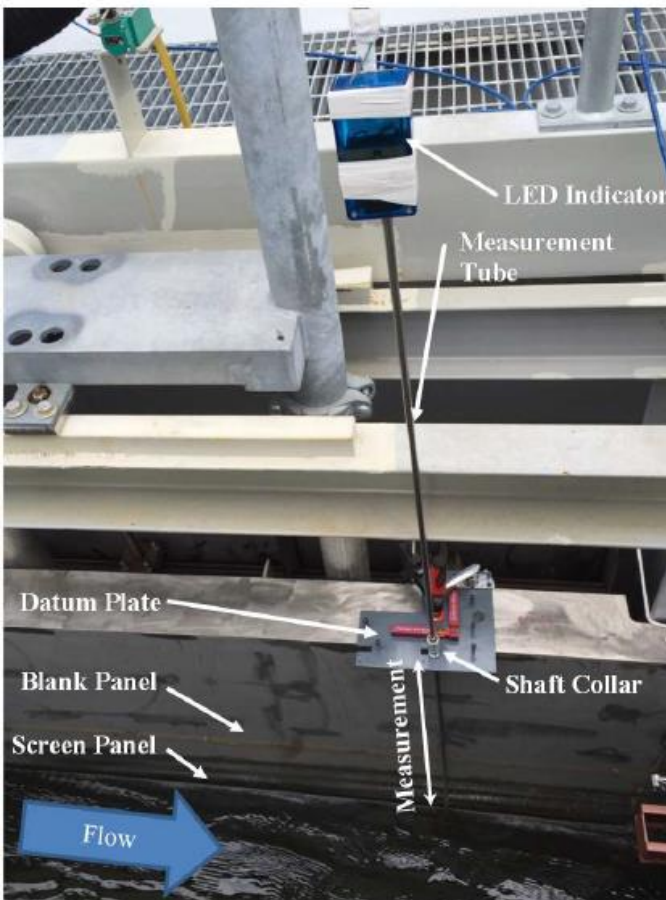
- Many Recent Designs Use High-Velocity Channel Flow to Trap Fish
- This Can Compromise Ability to Accurately Measure Approach Velocity

## Solution:

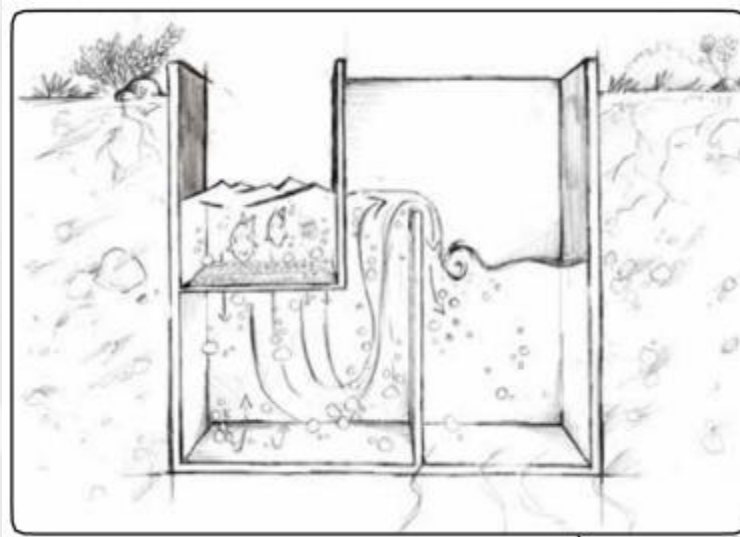
- Alternate Method to Reasonably Calculate Estimates of Approach Velocities

# Methods for Screen Head Measurements

## LED Water Surface Indicator Measurement Tool Used at North Fork FSC







Copied from <https://farmersscreen.org/>



